

DEVELOPMENT OF A FRAMEWORK FOR THE EVALUATION OF SEISMIC PERFORMANCE FACTORS FOR NON- STRUCTURAL ELEMENTS

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THE EVOLUTION OF THE SEISMIC DESIGN METHODOLOGIES AND BUILDING CODES



2003-08



2006-13



2016



2017



L'Aquila 2009



Emilia 2012



Centro Italia 2016

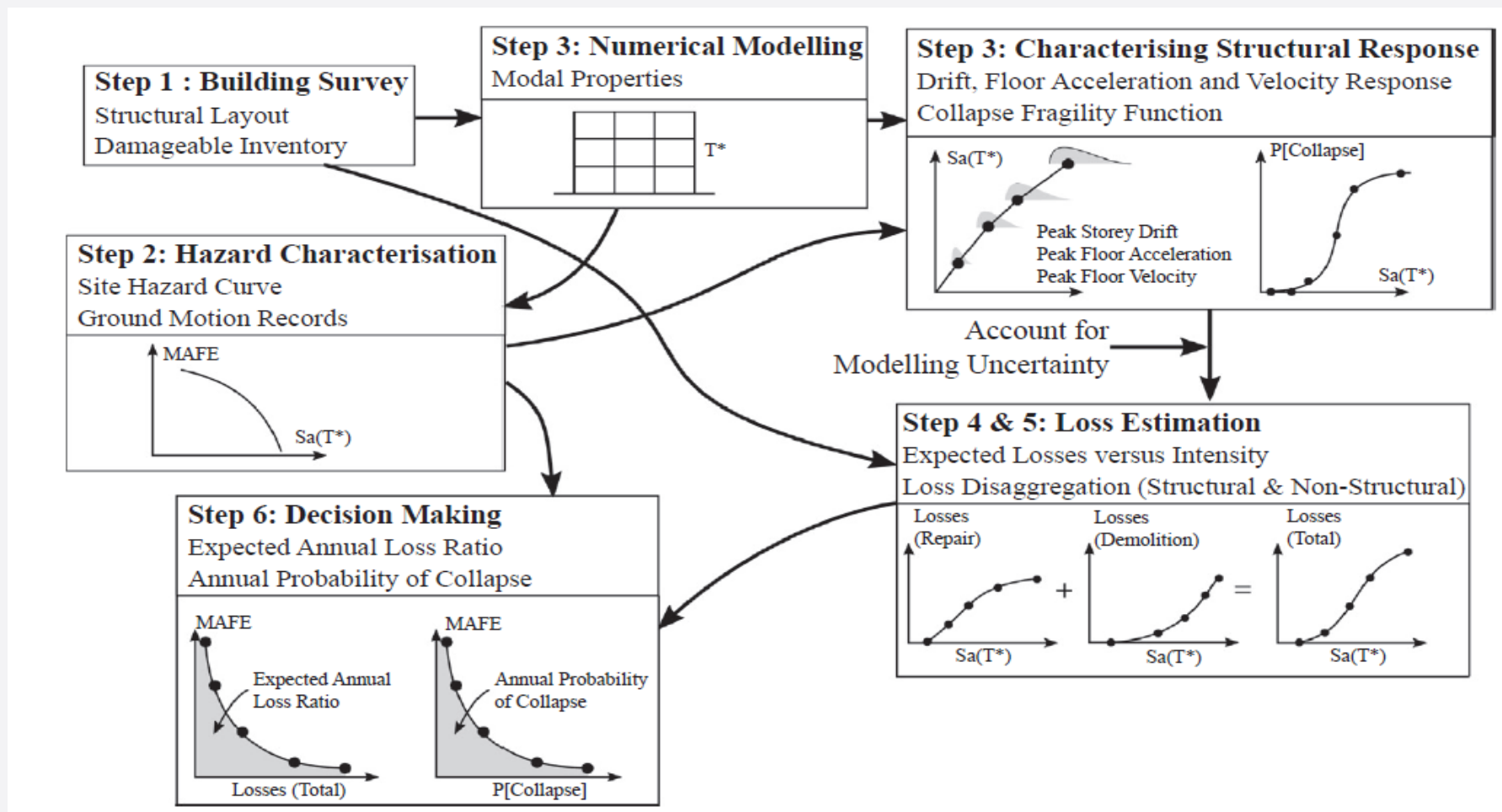


Ponte Morandi 2018

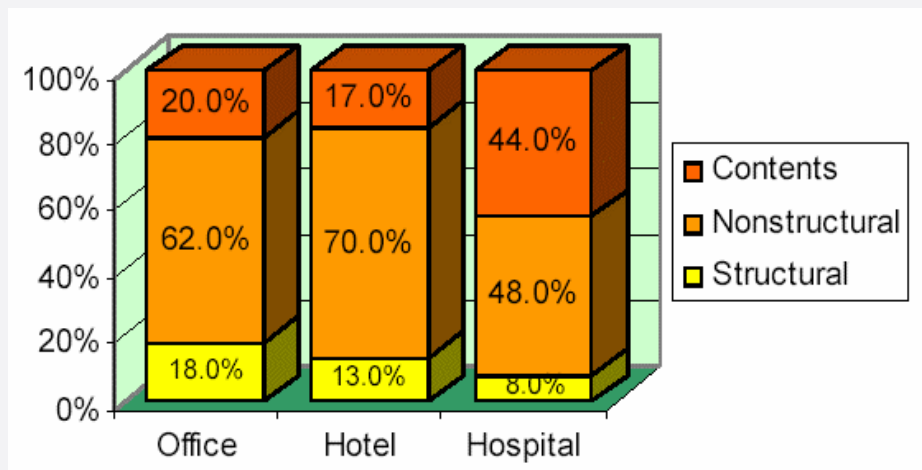


2018

THE PERFORMANCE-BASED EARTHQUAKE ENGINEERING



THE IMPORTANCE OF NON-STRUCTURAL ELEMENTS



ARE THE SEISMIC DESIGN AND QUALIFICATION METHODOLOGIES FOR NON-STRUCTURAL ELEMENTS WELL ESTABLISHED?

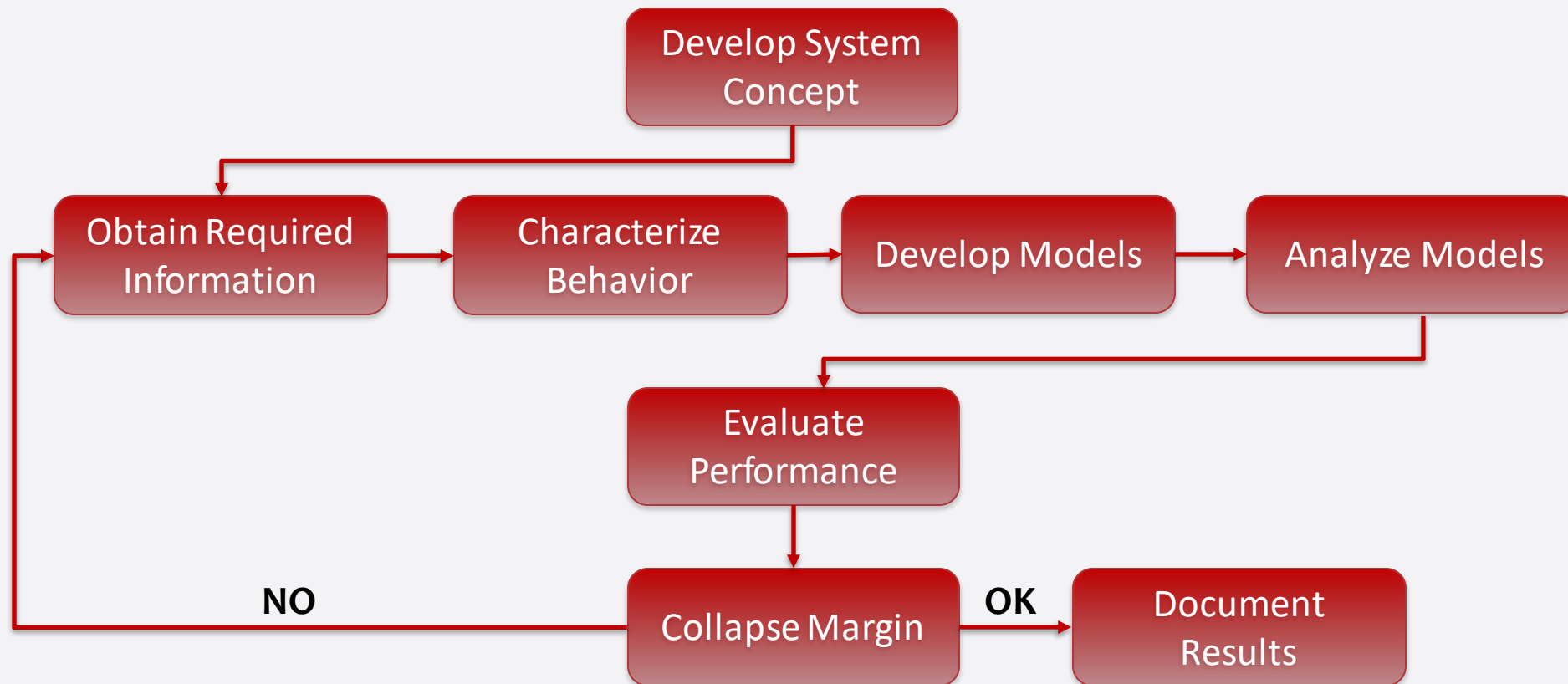


SOME SHORTCOMINGS

- 1. A methodology for quantifying the seismic performance of non-structural elements is missing;**
- 2. All the design procedures available in the international building codes account for force-based approaches, which are characterized by many shortcomings;**
- 3. The seismic qualification procedures still require some improvements and are not well established around the world.**

FEMA P695

QUANTIFICATION OF BUILDING SEISMIC PERFORMANCE FACTORS



FEMA P695

QUANTIFICATION OF BUILDING SEISMIC PERFORMANCE FACTORS

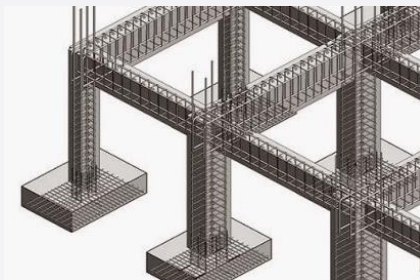
Obtain Required
Information

Identify Intended
Application

Establish Design
Requirements

Acquire Test
Data

Assess Quality of
Design Requirements
and Test Data



FEMA P695

QUANTIFICATION OF BUILDING SEISMIC PERFORMANCE FACTORS

Characterize
Behavior

Consider Configuration
Issues

Consider Behavioral
Effects

Develop Index Archetype
Configurations

Define Archetype Design
Space

Identify Performance Groups

Identify Design Variables

Table 4-1 Configuration Design Variables and Related Physical Properties

Design Variable	Related Physical Properties
Occupancy and Use	<ul style="list-style-type: none"> Typical framing layout Distribution of seismic-force-resisting system components Gravity load intensity Component overstrength
Elevation and Plan Configuration	<ul style="list-style-type: none"> Distribution of seismic-force-resisting components Typical framing layout Permitted vertical (strength and stiffness) irregularities Beam spans, number of framing bays, system regularity Wall length, aspect ratio, plan geometry, wall coupling Braced bay size, number of braced bays, bracing configuration Diaphragm proportions, strength, and stiffness (or flexibility) Ratio of seismic mass to seismic-force-resisting components Ratio of tributary gravity load to seismic load
Building Height	<ul style="list-style-type: none"> Story heights Number of stories
Structural Component Type	<ul style="list-style-type: none"> Moment frame connection types Bracing component types Shear wall sheathing and fastener types Isolator properties and types
Seismic Design Category	<ul style="list-style-type: none"> Design ground motion intensity Special design/detailing requirements Application limits
Gravity Load	<ul style="list-style-type: none"> Gravity load intensity Typical framing layout Ratio of tributary gravity load to seismic load Component overstrength

Identify Performance Group

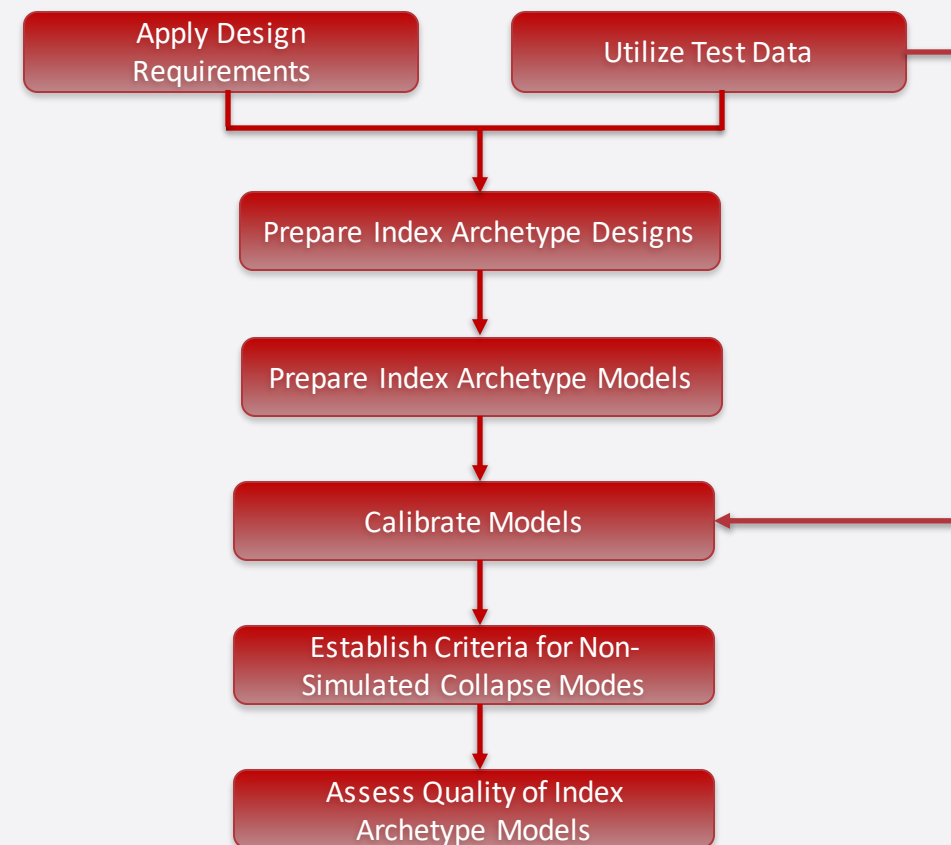
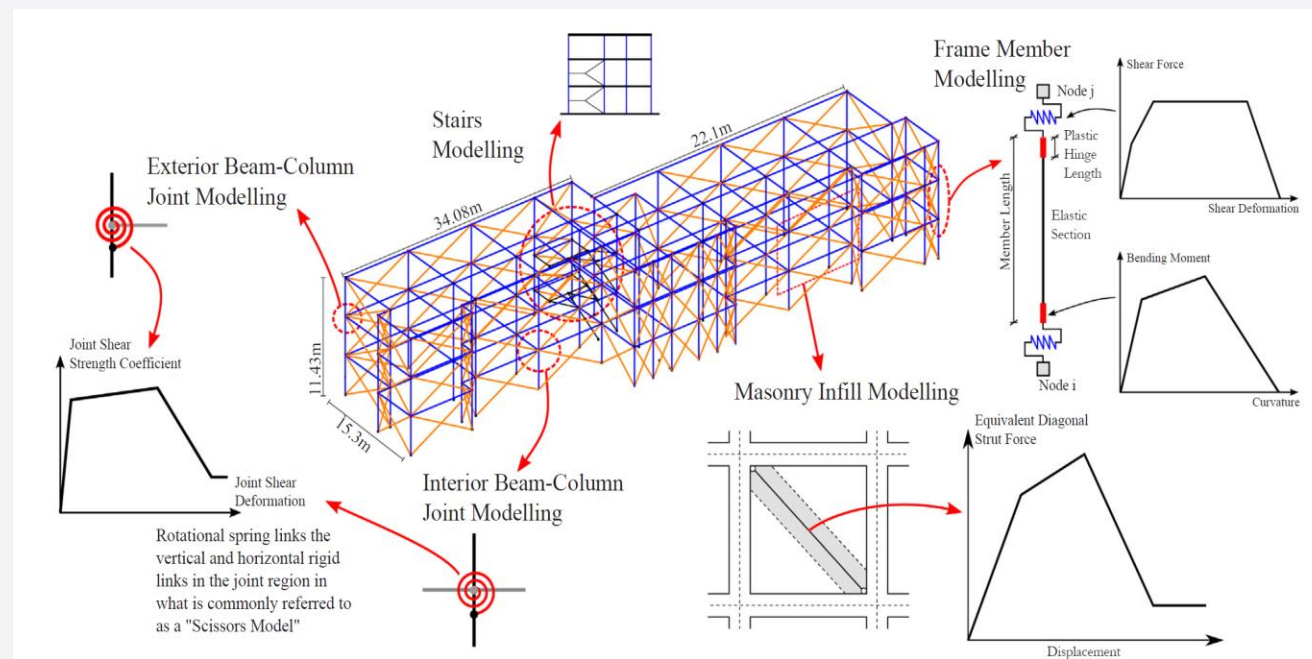
Table 4-3 Generic Performance Group Matrix

Performance Group Summary					
Group No.	Basic Configuration	Design Load Level		Period Domain	Number of Archetypes
		Gravity	Seismic		
PG-1	Type 1	High	Max SDC	Short	≥ 3
PG-2				Long	≥ 3
PG-3			Min SDC	Short	≥ 3
PG-4		Low		Long	≥ 3
PG-5			Max SDC	Short	≥ 3
PG-6				Long	≥ 3
PG-7	Type 2	High	Min SDC	Short	≥ 3
PG-8				Long	≥ 3
PG-9		Low	Max SDC	Short	≥ 3
PG-10				Long	≥ 3
PG-11		High	Min SDC	Short	≥ 3
PG-12				Long	≥ 3
PG-13	Type N	Low	Max SDC	Short	≥ 3
PG-14				Long	≥ 3
PG-15		High	Min SDC	Short	≥ 3
PG-16				Long	≥ 3
PG-17		Low	Max SDC	Short	≥ 3
PG-18				Long	≥ 3
PG-19	Type N	High	Min SDC	Short	≥ 3
PG-20				Long	≥ 3
PG-21		Low	Max SDC	Short	≥ 3
PG-22				Long	≥ 3
PG-23		High	Min SDC	Short	≥ 3
PG-24				Long	≥ 3

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QUANTIFICATION OF BUILDING SEISMIC PERFORMANCE FACTORS

Develop Models



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QUANTIFICATION OF BUILDING SEISMIC PERFORMANCE FACTORS

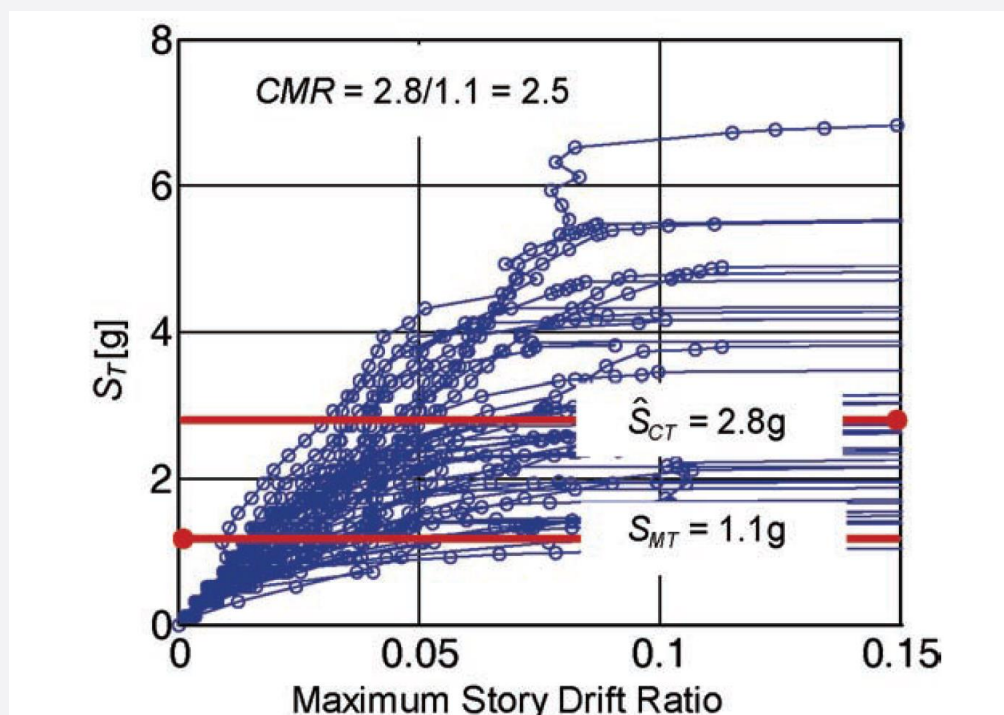
Analyze Models

Perform Nonlinear Static
Analysis

Perform Nonlinear Dynamic
Analyses

Calculate:

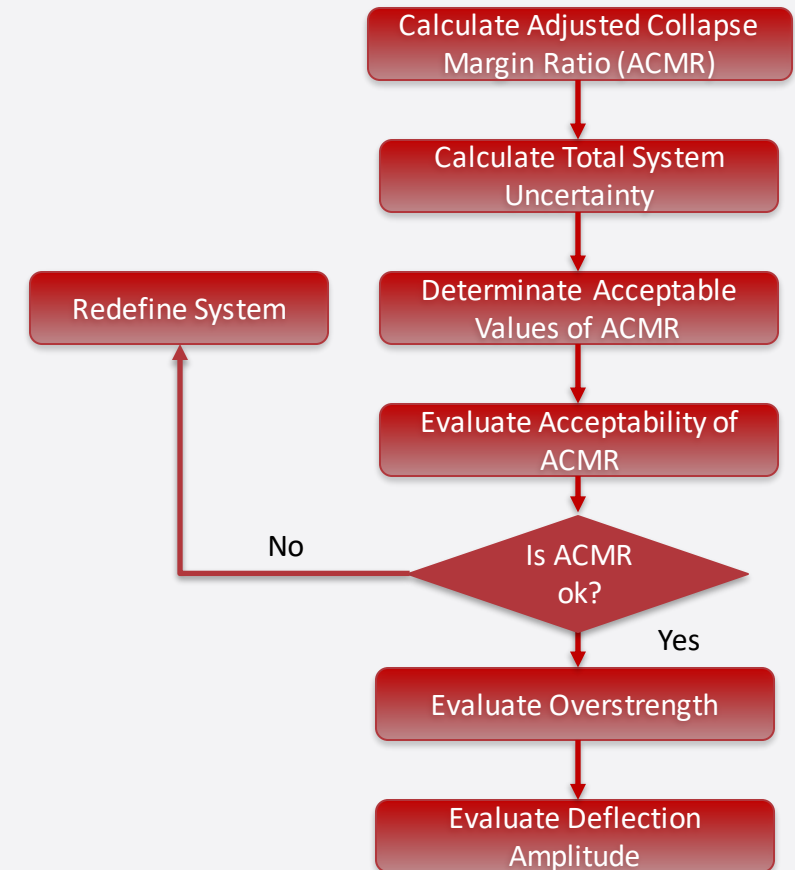
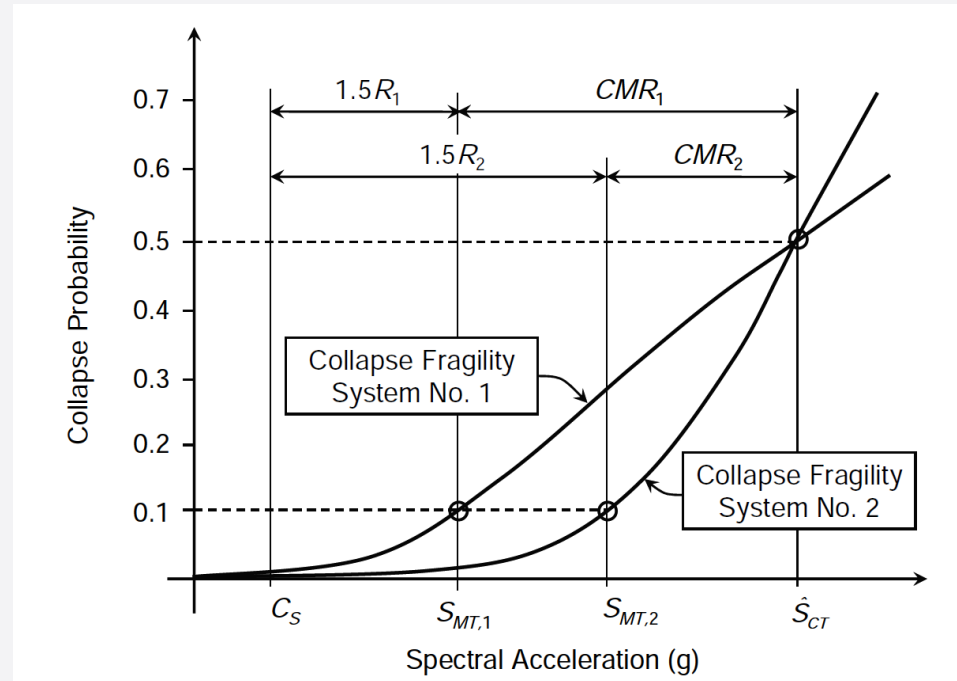
- Period-based ductility
- Median Collapse Intensity
- Collapse Margin Ratio



FEMA P695

QUANTIFICATION OF BUILDING SEISMIC PERFORMANCE FACTORS

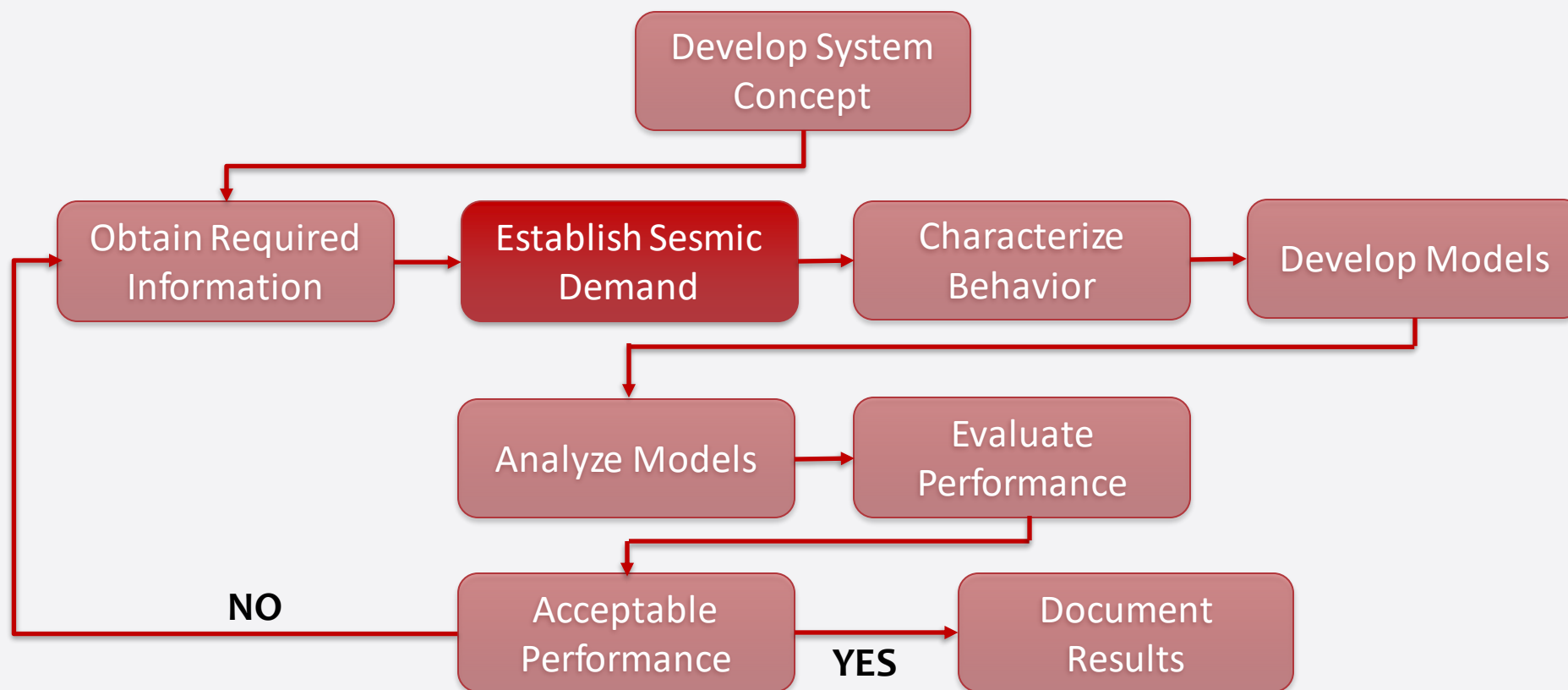
Evaluate
Performance



IS THE FEMA P695 APPLICABLE FOR QUANTIFYING PERFORMANCE PARAMETERS FOR NON- STRUCTURAL ELEMENTS?

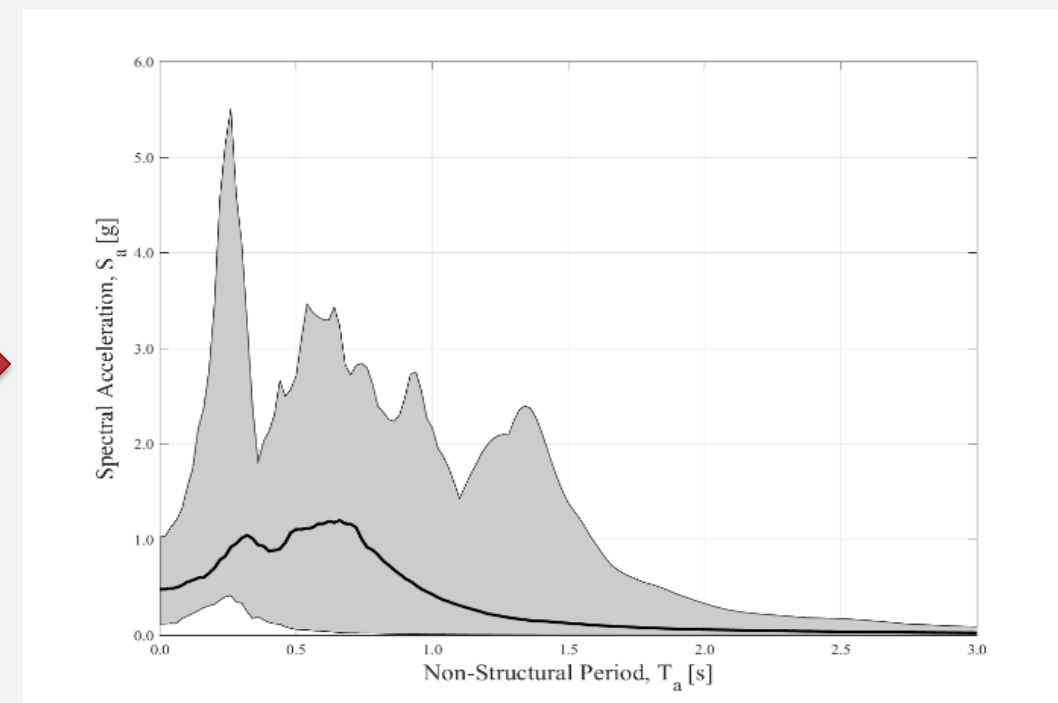
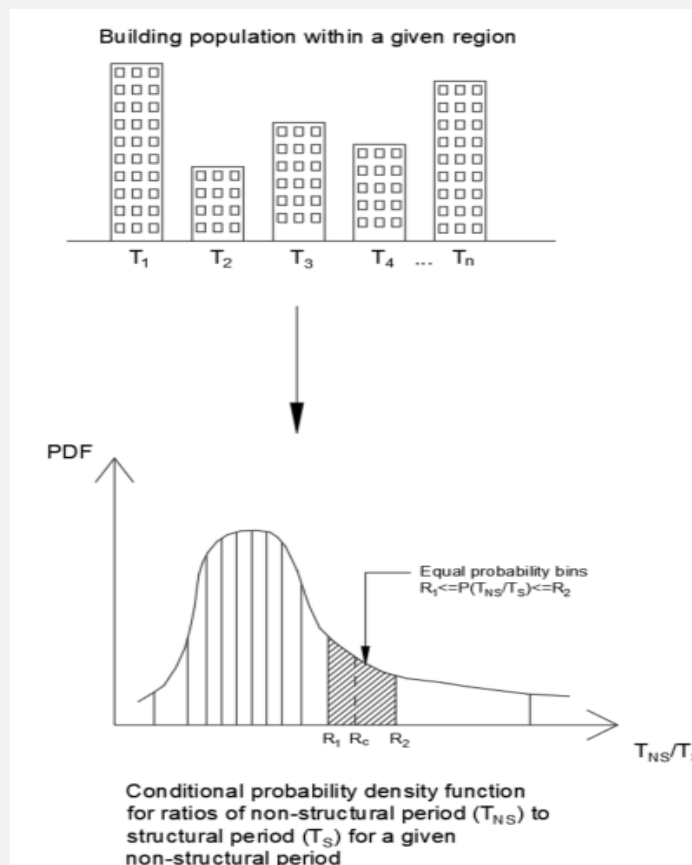


PROPOSED FRAMEWORK FOR THE QUANTIFICATION OF SEISMIC PERFORMANCE FACTOR FOR NON-STRUCTURAL BUILDING ELEMENTS



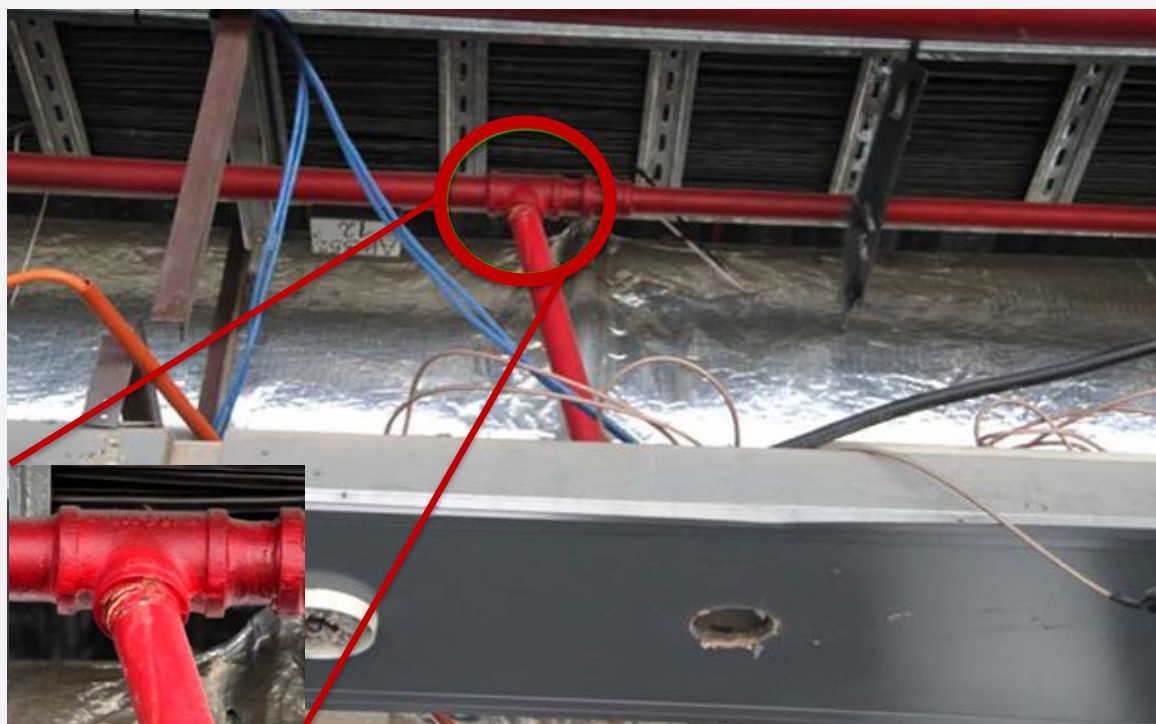
PROPOSED FRAMEWORK FOR THE QUANTIFICATION OF SEISMIC PERFORMANCE FACTOR FOR NON-STRUCTURAL BUILDING ELEMENTS

Establish Seismic Demand

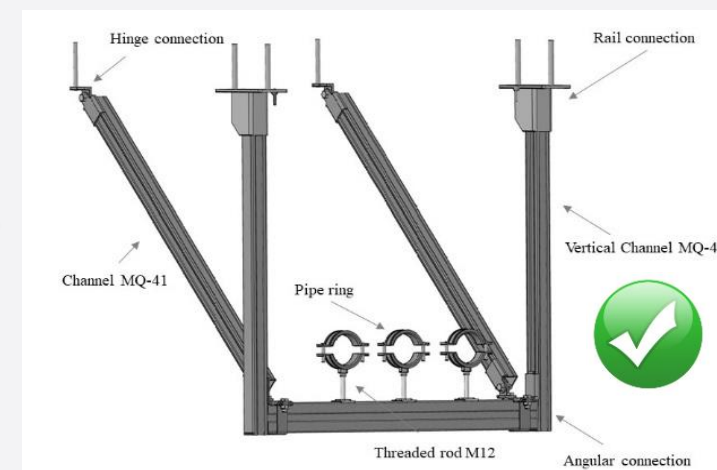


ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

TYPICAL DAMAGE



HOW TO AVOID



THE QUESTION

$$F_a = \frac{S_a \gamma_a}{q_a} W_a \quad ?$$

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Obtain Required
Information

Establish Design
Requirements

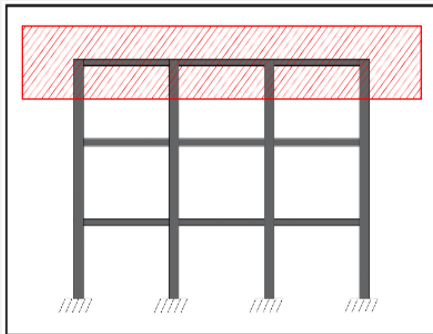
THE EUROCODE 8 DESIGN APPROACH HAS BEEN CONSIDERED FOR THE DESIGN OF
THE ARCHETYPES

$$F_a = \frac{S_a \gamma_a}{q_a} W_a$$

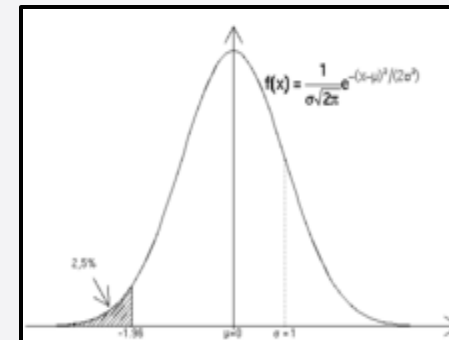
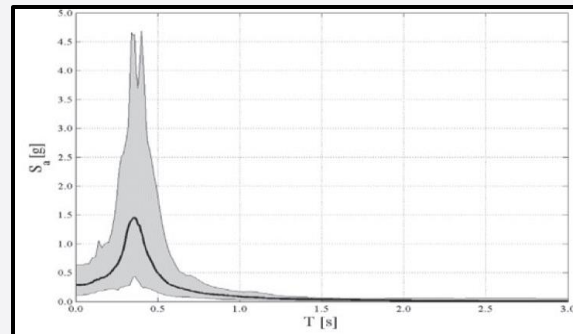
$$S_a = a_g S \left(\frac{3(1 + z/H)}{1 + (1 - T_a/T_n)^2} - 0.5 \right)$$

$$F_a \leq \frac{F_{Rk}}{\gamma_m}$$

z/H



T_a/T_n



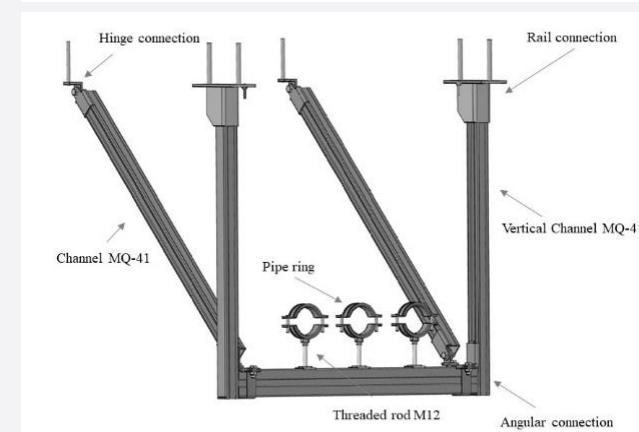
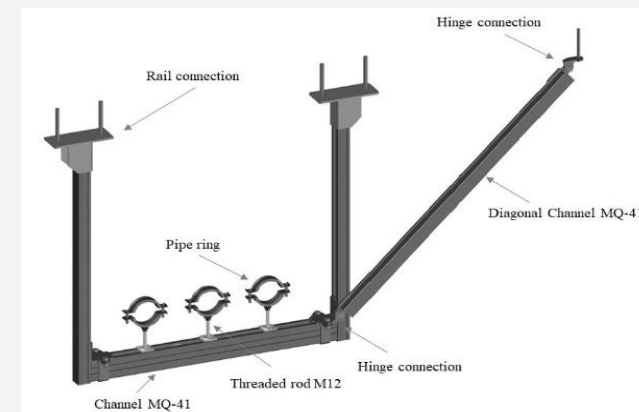
**ADDITIONAL
PRESCRIPTIVE RULES FOR
SPACING**

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Obtain Required
Information



Test data

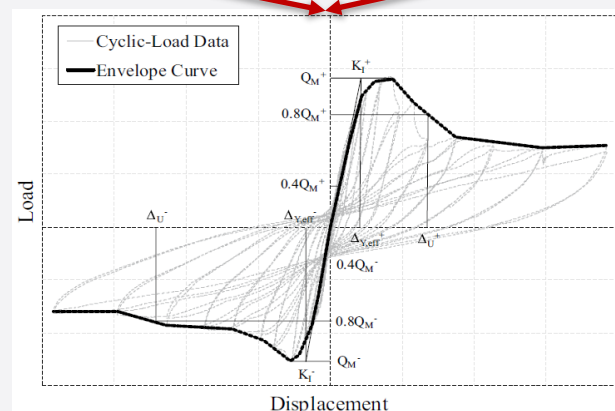
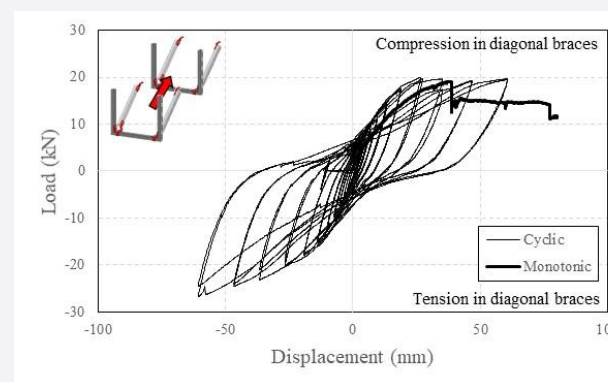
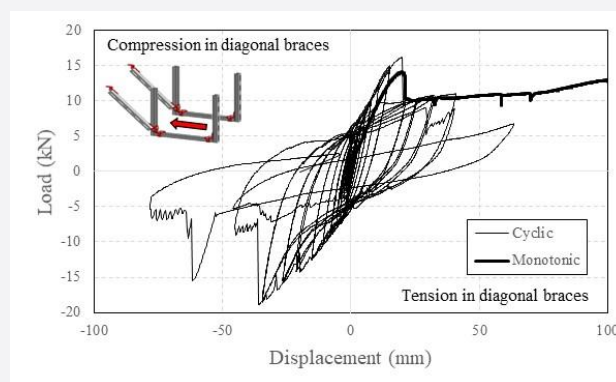


ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Obtain Required Information



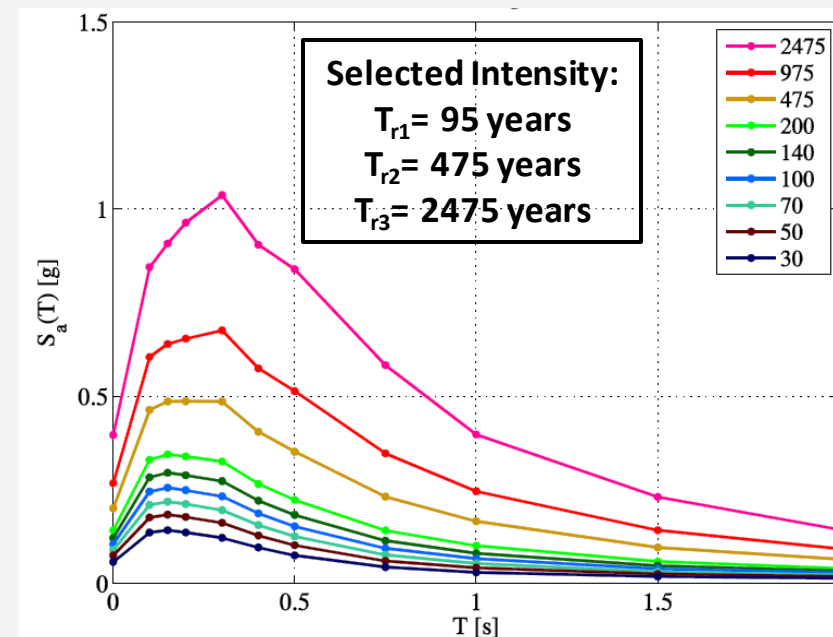
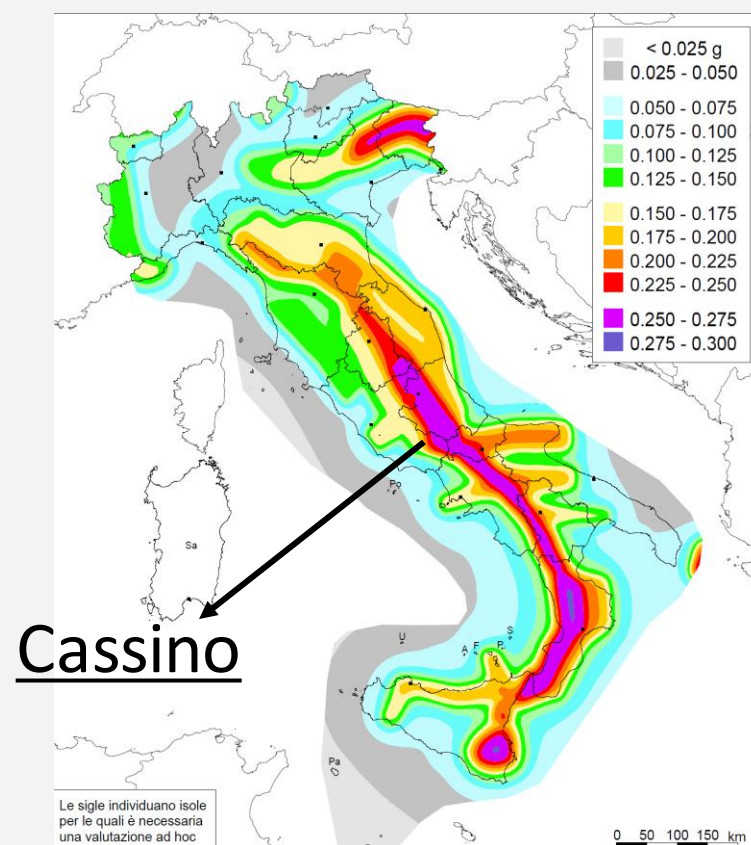
Test data



Suspended piping restraint installation	Q_M (kN)	$\Delta_{Y,eff}$ (mm)	Δ_U (mm)	μ_{eff}
Transverse	15.79	13.12	24.87	1.9
Longitudinal	22.08	17.32	53.46	3.1

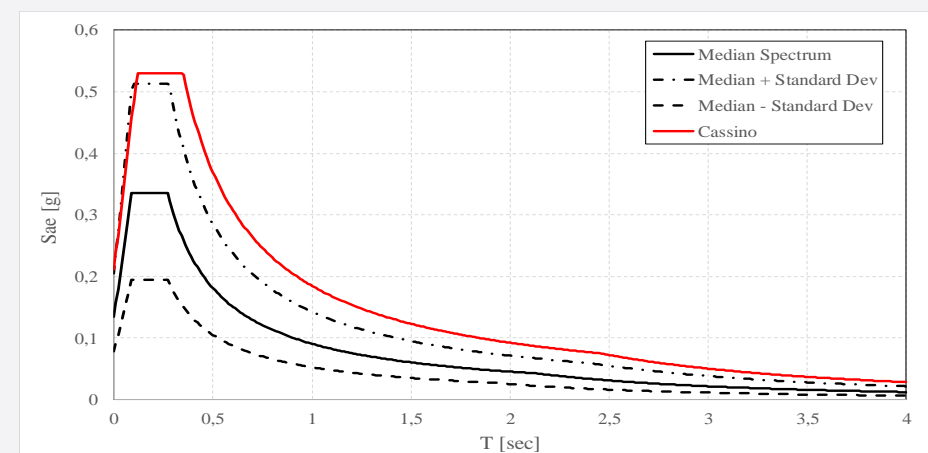
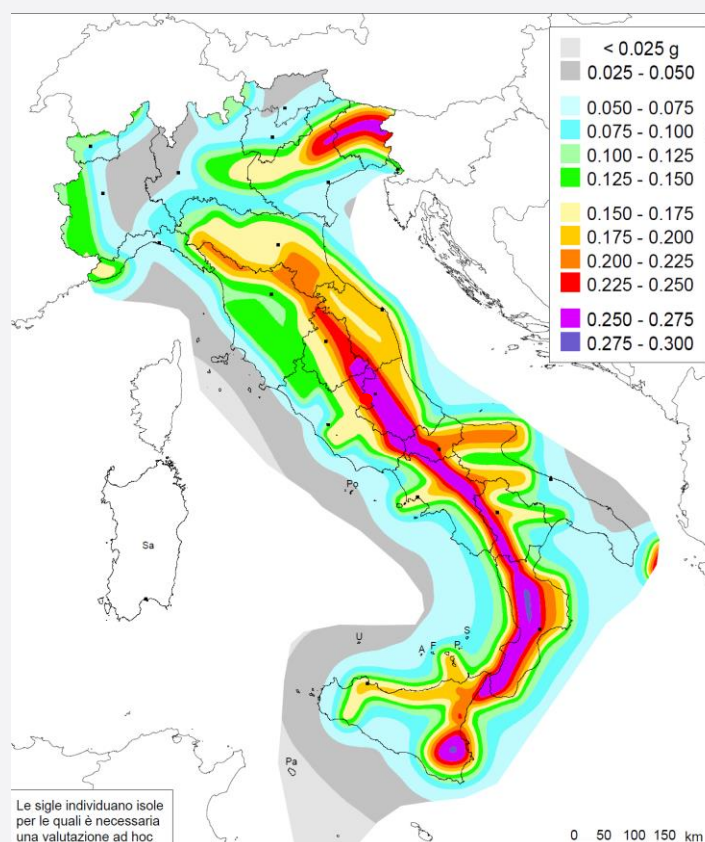
ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Establish Seismic Demand

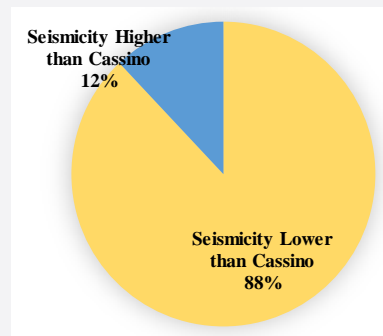


ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

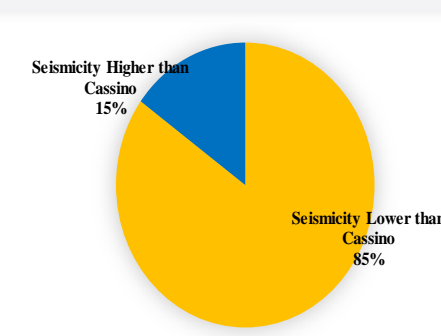
Establish Seismic
Demand



Italy

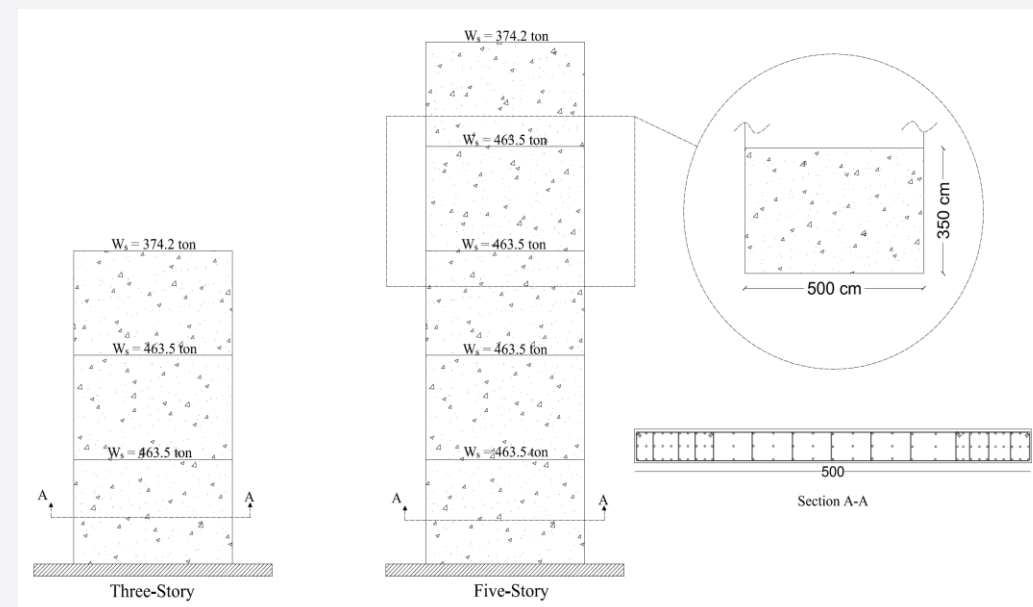
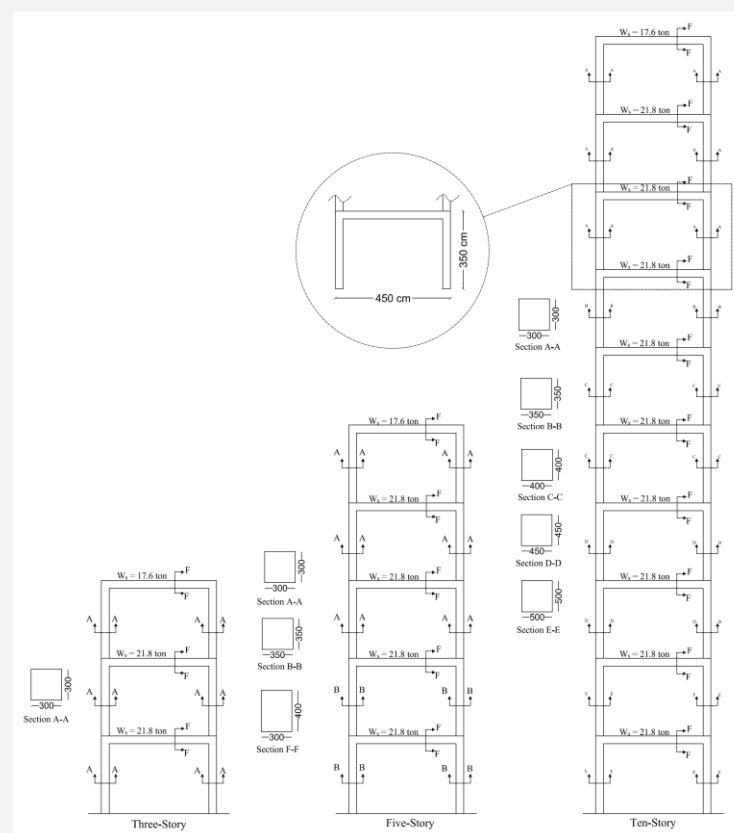


Europe



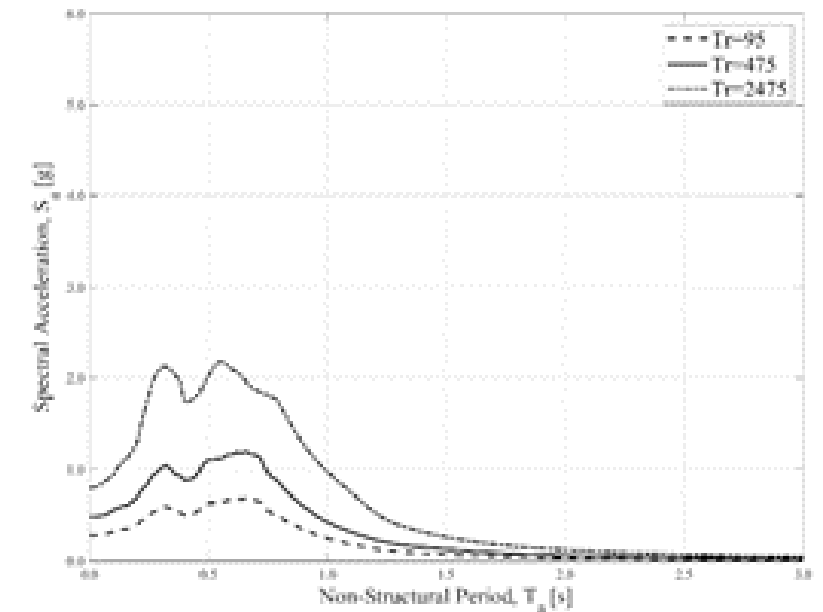
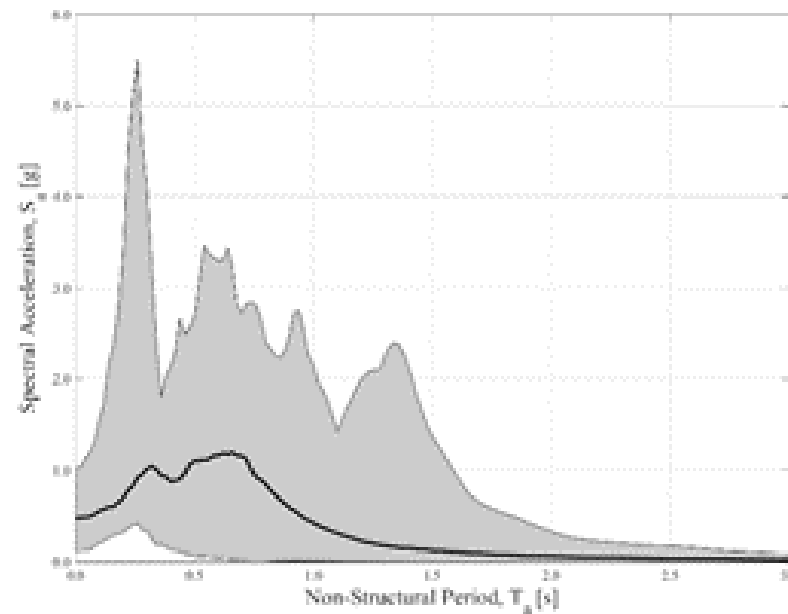
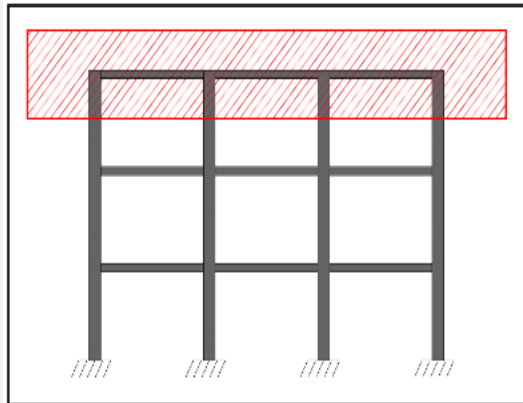
ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Establish Seismic
Demand



ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Establish Seismic
Demand

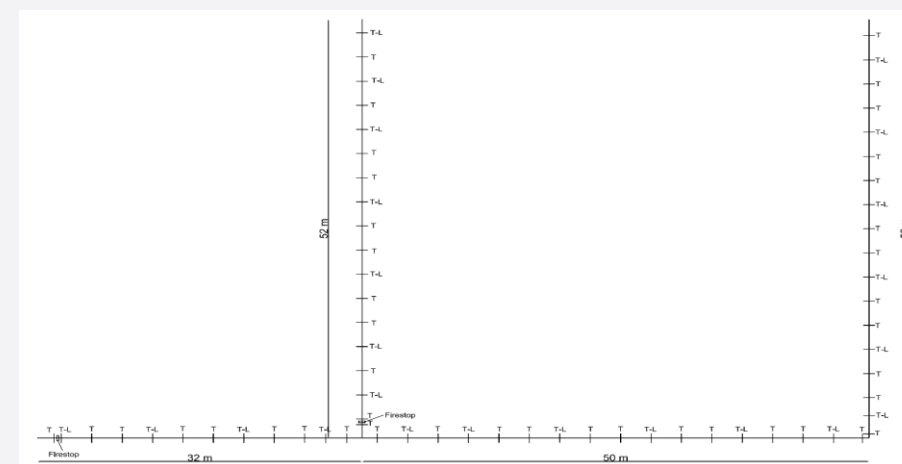
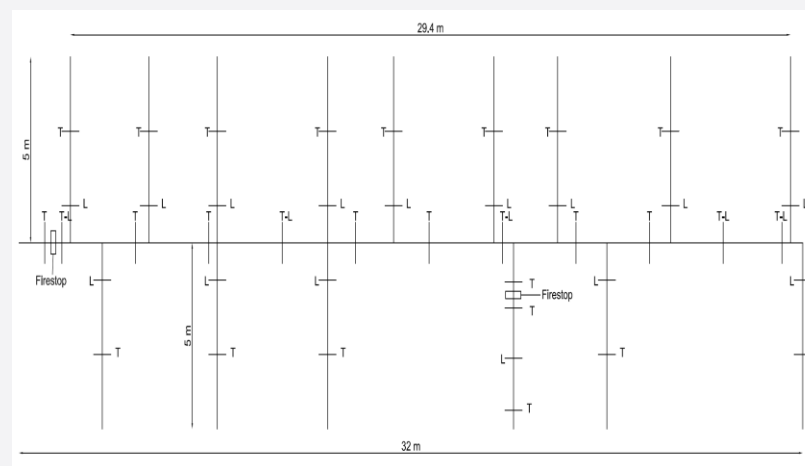


ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Characterize
Behavior

Define Archetype
representative of the
design space

1. Geometrical
configuration

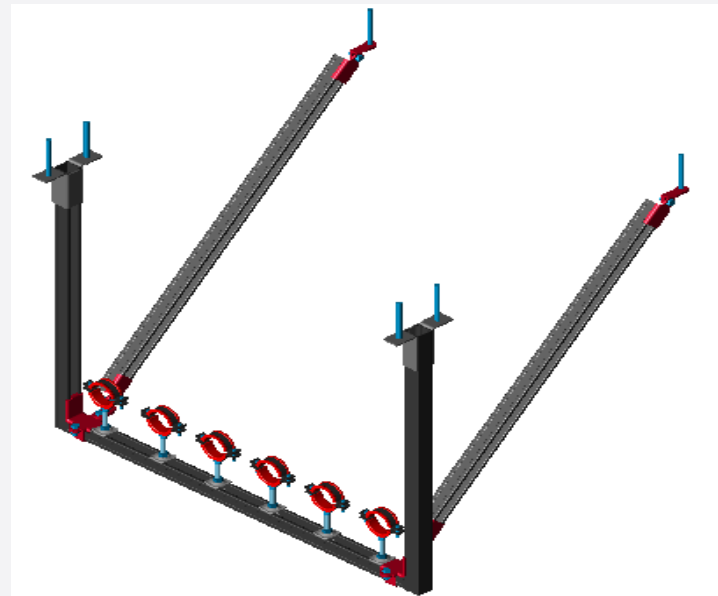


ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

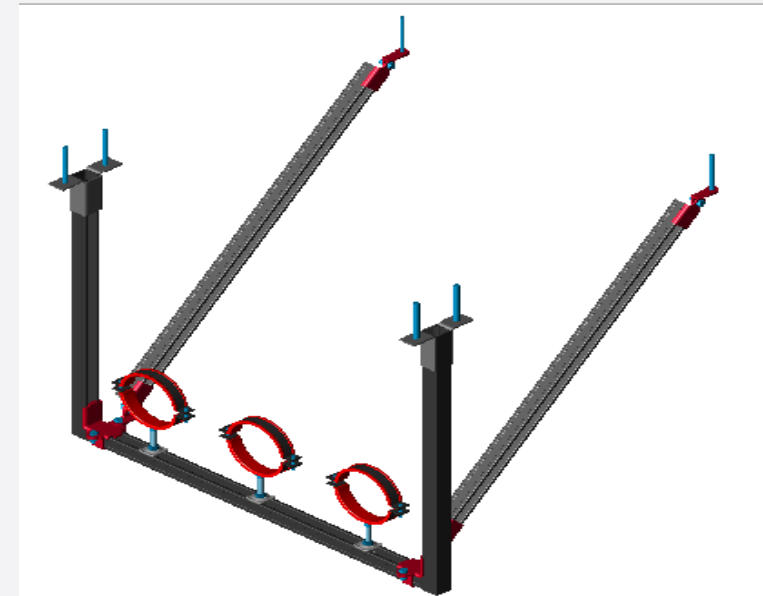
Characterize
Behavior

Define Archetype
representative of the
design space

2. Number of pipes
and material



Six composite Mepla pipes with a
diameter equal to 50 mm



Three steel pipes with a
diameter equal to 127 mm

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Characterize
Behavior



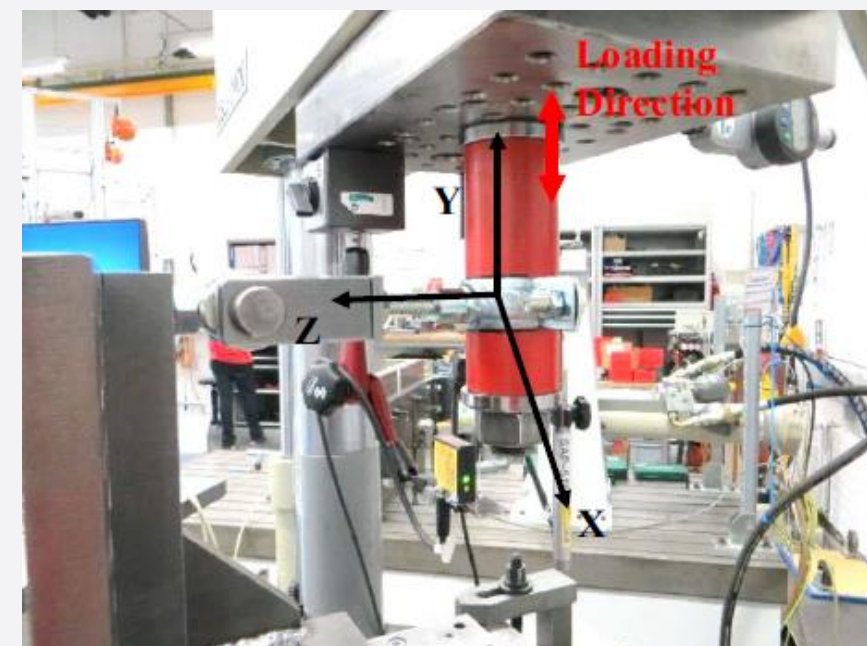
Define Archetype
representative of the
design space



3. Pipe ring
typology



Stiff pipe ring



Soft pipe ring

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Characterize
Behavior

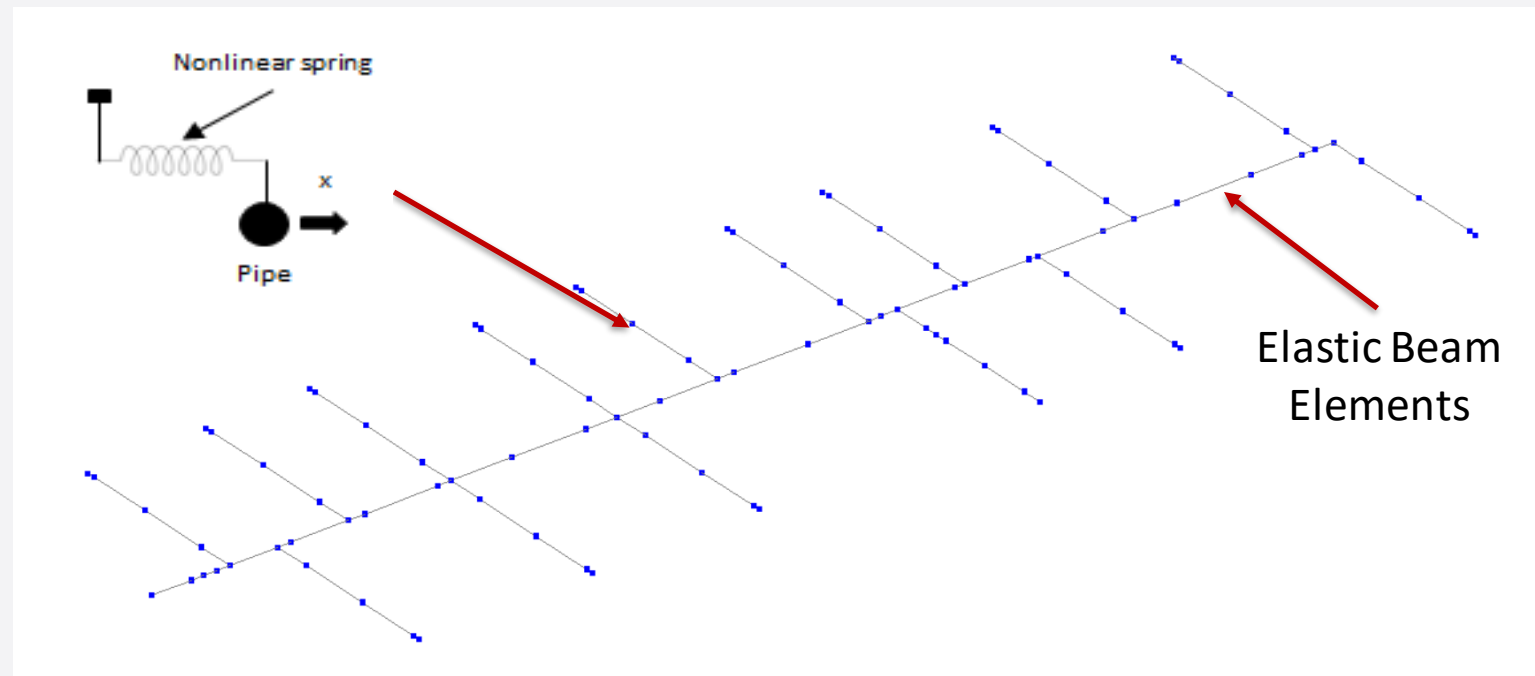


Define Archetype
representative of the
design space

Key archetype design parameters					
Archetype ID	Geometry	Pipe material	Pipe diameter (mm)	Pipe ring typology	Q_s
Performance group PG-1					
1	W-B	Composite Mepla	50	Soft pipe ring	1
2	W-B	Composite Mepla	50	Soft pipe ring	2
3	W-B	Composite Mepla	50	Soft pipe ring	4
Performance group PG-2					
4	W-B	Composite Mepla	50	Stiff pipe ring	1
5	W-B	Composite Mepla	50	Stiff pipe ring	2
6	W-B	Composite Mepla	50	Stiff pipe ring	4
Performance group PG-3					
7	W-B	Steel pipe	127	Soft pipe ring	1
8	W-B	Steel pipe	127	Soft pipe ring	2
9	W-B	Steel pipe	127	Soft pipe ring	4
Performance group PG-4					
10	W-B	Steel pipe	127	Stiff pipe ring	1
11	W-B	Steel pipe	127	Stiff pipe ring	2
12	W-B	Steel pipe	127	Stiff pipe ring	4
Performance group PG-5					
13	WO-B	Composite Mepla	50	Soft pipe ring	1
14	WO-B	Composite Mepla	50	Soft pipe ring	2
15	WO-B	Composite Mepla	50	Soft pipe ring	4
Performance group PG-6					
16	WO-B	Composite Mepla	50	Stiff pipe ring	1
17	WO-B	Composite Mepla	50	Stiff pipe ring	2
18	WO-B	Composite Mepla	50	Stiff pipe ring	4
Performance group PG-7					
19	WO-B	Steel pipe	127	Soft pipe ring	1
20	WO-B	Steel pipe	127	Soft pipe ring	2
21	WO-B	Steel pipe	127	Soft pipe ring	4
Performance group PG-8					
22	WO-B	Steel pipe	127	Stiff pipe ring	1
23	WO-B	Steel pipe	127	Stiff pipe ring	2
24	WO-B	Steel pipe	127	Stiff pipe ring	4

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

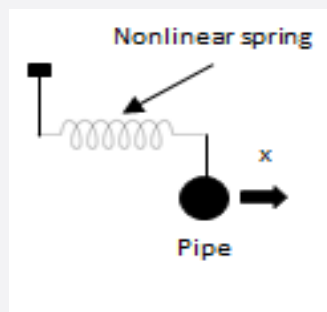
Develop Models



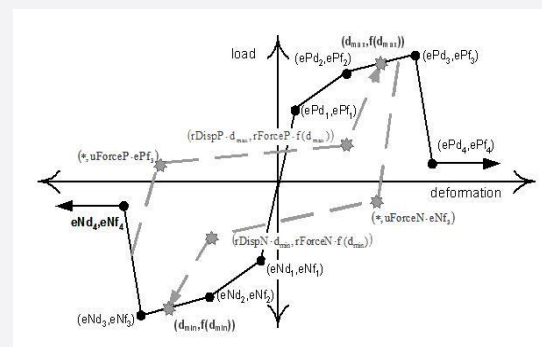
ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Develop Models

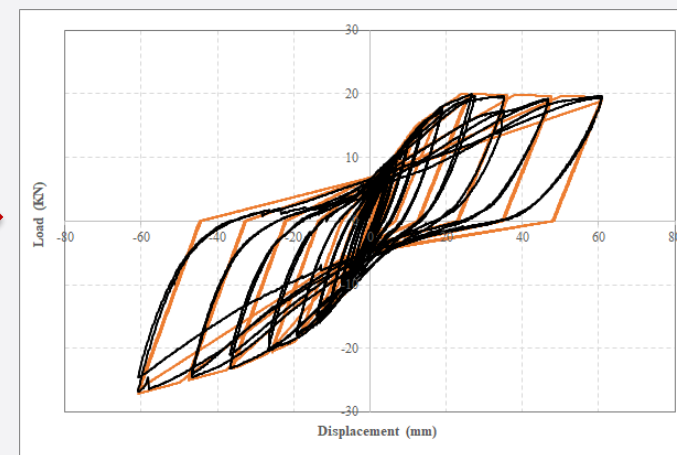
OpenSees



Numerical
Approach



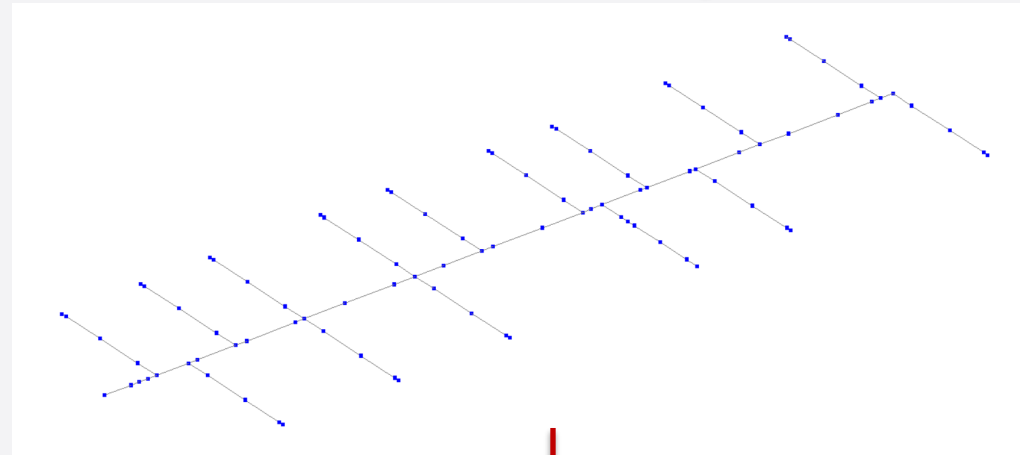
Tool:
Pinching4Material
OpenSees



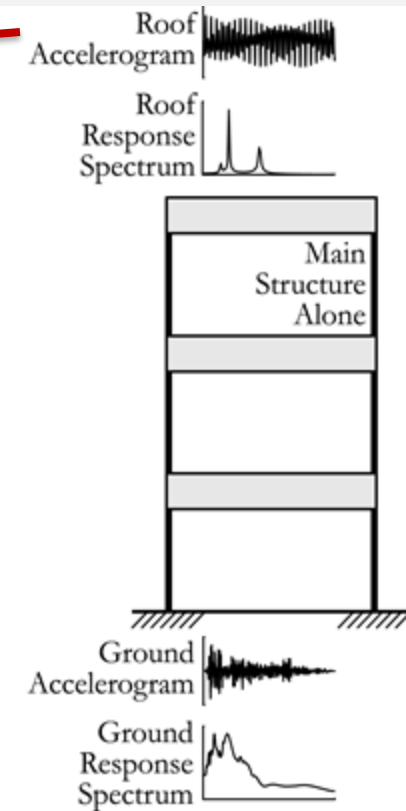
Numerical VS
Experimental

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Analyze Models



ENGINEERING DEMAND PARAMETERS: **DUCTILITY DEMAND**



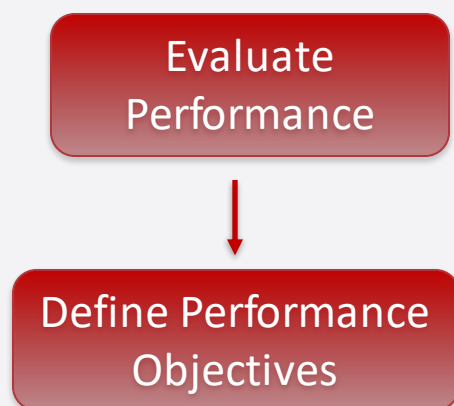
ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Analyze Models

ENGINEERING DEMAND
PARAMETERS:
DUCTILITY DEMAND

Archetype ID	q_a	Ductility demand											
		$T_r = 95$ years				$T_r = 475$ years				$T_r = 2475$ years			
		Transverse		Longitudinal		Transverse		Longitudinal		Transverse		Longitudinal	
		m	m+ s	m	m+ s	m	m+ s	m	m+ s	m	m+ s	m	m+ s
Performance group PG-1													
1	1	0.49	0.55	0.21	0.29	0.63	0.85	0.30	0.47	0.80	1.27	0.39	0.61
2	2	0.49	0.55	0.21	0.29	0.63	0.85	0.30	0.47	0.80	1.27	0.39	0.61
3	4	0.49	0.55	0.21	0.29	0.63	0.85	0.30	0.47	0.80	1.27	0.39	0.61
Performance group PG-2													
4	1	0.48	0.55	0.22	0.30	0.64	0.84	0.32	0.51	0.79	1.26	0.43	0.74
5	2	0.48	0.55	0.22	0.30	0.64	0.84	0.32	0.51	0.79	1.26	0.43	0.74
6	4	0.48	0.55	0.22	0.30	0.64	0.84	0.32	0.51	0.79	1.26	0.43	0.74
Performance group PG-3													
7	1	0.33	0.42	0.17	0.25	0.40	0.70	0.24	0.40	0.76	1.15	0.44	0.64
8	2	0.34	0.49	0.18	0.30	0.51	0.86	0.26	0.53	0.84	1.10	0.50	0.64
9	4	0.40	0.54	0.24	0.33	0.51	0.86	0.32	0.58	0.91	1.00	0.54	0.79
Performance group PG-4													
10	1	0.34	0.45	0.18	0.26	0.51	0.81	0.30	0.48	0.76	1.10	0.45	0.67
11	2	0.37	0.49	0.20	0.26	0.55	0.86	0.30	0.52	0.84	1.24	0.53	0.72
12	4	0.41	0.57	0.24	0.34	0.52	0.92	0.33	0.59	0.90	1.33	0.54	0.77
Performance group PG-5													
13	1	0.47	0.71	0.15	0.25	0.62	0.90	0.28	0.44	0.84	1.26	0.45	0.67
14	2	0.47	0.71	0.15	0.25	0.62	0.90	0.28	0.44	0.84	1.26	0.45	0.67
15	4	0.47	0.71	0.15	0.25	0.62	0.90	0.28	0.44	0.84	1.26	0.45	0.67
Performance group PG-6													
16	1	0.47	0.61	0.17	0.27	0.64	0.96	0.30	0.50	0.77	1.24	0.49	0.71
17	2	0.47	0.61	0.17	0.27	0.64	0.96	0.30	0.50	0.77	1.24	0.49	0.71
18	4	0.47	0.61	0.17	0.27	0.64	0.96	0.30	0.50	0.77	1.24	0.49	0.71
Performance group PG-7													
19	1	0.40	0.53	0.16	0.26	0.56	1.05	0.27	0.52	1.02	1.30	0.46	0.71
20	2	0.35	0.75	0.14	0.42	0.56	1.45	0.36	0.73	1.16	1.51	0.68	0.90
21	4	0.48	0.87	0.22	0.57	0.79	2.24	0.40	1.11	1.59	2.68	1.02	1.26
Performance group PG-8													
22	1	0.42	0.59	0.17	0.30	0.60	1.12	0.30	0.55	0.91	1.45	0.52	0.77
23	2	0.44	0.63	0.22	0.36	0.58	1.53	0.28	0.97	1.12	1.57	0.70	0.92
24	4	0.48	0.87	0.24	0.60	0.84	1.75	0.42	1.11	1.76	2.20	1.16	1.39

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS



Performance groups	Sway braced trapeze performance parameters				
	Bracing direction	Q_M (kN)	$\Delta_{Y,eff}$ (mm)	Δ_U (mm)	μ_{eff}
PG1 and PG5	Longitudinal	14.81	34.78	176.33	5.1
	Transverse	6.96	19.97	25.95	1.3
PG2 and PG6	Longitudinal	14.81	31.28	80.28	2.6
	Transverse	6.96	20.21	26.70	1.3
PG3 and PG7	Longitudinal	15.56	36.60	170.30	4.7
	Transverse	6.96	21.79	67.60	3.1
PG4 and PG8	Longitudinal	15.56	34.00	80.00	2.4
	Transverse	6.96	20.11	47.91	2.4

ACHIEVEMENT OF PERFORMANCE OBJECTIVES

DAMAGE LIMITATION PERFORMANCE OBJECTIVE: **EFFECTIVE DUCTILITY CAPACITY (μ_{eff})=1.0**

LIFE SAFETY PERFORMANCE OBJECTIVE: **EFFECTIVE DUCTILITY CAPACITY (μ_{eff})= $\Delta_U/\Delta_{Y,eff}$**

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Evaluate
Performance

ALL ARCHETYPES PASS
THE PERFORMANCE
EVALUATION



For this case study, $q_a = 4$
is adequate for the
forced-based seismic
design of sway braced
trapezes

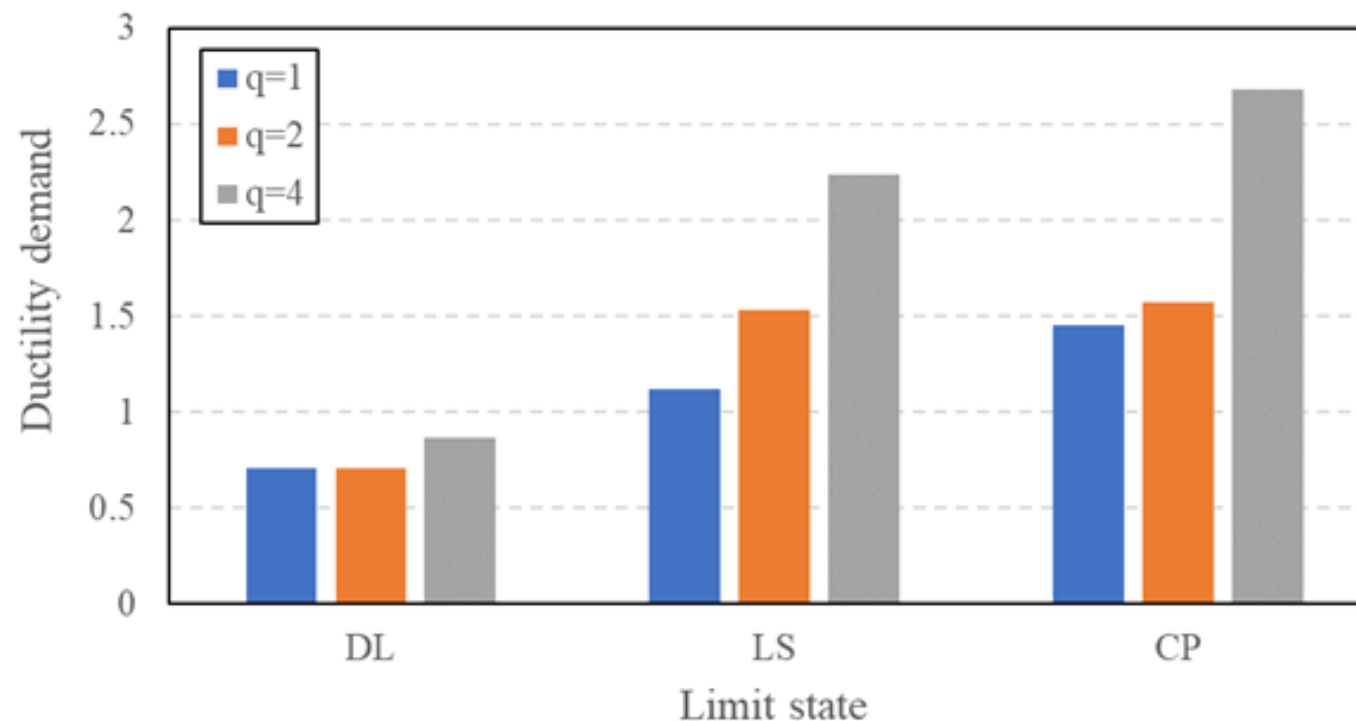
Archetype ID	q _a	Pass/Fail criterion											
		T _r = 95 years				T _r = 475 years				T _r = 2475 years			
		Transverse		Longitudinal		Transverse		Longitudinal		Transverse		Longitudinal	
		$\frac{\mu + \sigma}{\mu_{eff}}$	Pass/Fail Criterion	$\frac{\mu + \sigma}{\mu_{eff}}$	Pass/Fail Criterion	$\frac{\mu + \sigma}{\mu_{eff}}$	Pass/Fail Criterion	$\frac{\mu + \sigma}{\mu_{eff}}$	Pass/Fail Criterion	$\frac{\mu + \sigma}{\mu_{eff}}$	Pass/Fail Criterion	$\frac{\mu + \sigma}{\mu_{eff}}$	Pass/Fail Criterion
Performance group PG-1													
1	1	0.55	Pass	0.29	Pass	0.66	Pass	0.09	Pass	0.98	Pass	0.12	Pass
2	2	0.55	Pass	0.29	Pass	0.66	Pass	0.09	Pass	0.98	Pass	0.12	Pass
3	4	0.55	Pass	0.29	Pass	0.66	Pass	0.09	Pass	0.98	Pass	0.12	Pass
Performance group PG-2													
4	1	0.55	Pass	0.30	Pass	0.65	Pass	0.20	Pass	0.97	Pass	0.29	Pass
5	2	0.55	Pass	0.30	Pass	0.65	Pass	0.20	Pass	0.97	Pass	0.29	Pass
6	4	0.55	Pass	0.30	Pass	0.65	Pass	0.20	Pass	0.97	Pass	0.29	Pass
Performance group PG-3													
7	1	0.42	Pass	0.25	Pass	0.22	Pass	0.08	Pass	0.37	Pass	0.14	Pass
8	2	0.49	Pass	0.30	Pass	0.28	Pass	0.11	Pass	0.35	Pass	0.14	Pass
9	4	0.54	Pass	0.33	Pass	0.28	Pass	0.12	Pass	0.32	Pass	0.14	Pass
Performance group PG-4													
10	1	0.45	Pass	0.26	Pass	0.34	Pass	0.20	Pass	0.46	Pass	0.28	Pass
11	2	0.49	Pass	0.26	Pass	0.36	Pass	0.22	Pass	0.52	Pass	0.30	Pass
12	4	0.57	Pass	0.34	Pass	0.38	Pass	0.25	Pass	0.55	Pass	0.32	Pass
Performance group PG-5													
13	1	0.71	Pass	0.25	Pass	0.69	Pass	0.09	Pass	0.97	Pass	0.13	Pass
14	2	0.71	Pass	0.25	Pass	0.69	Pass	0.09	Pass	0.97	Pass	0.13	Pass
15	4	0.71	Pass	0.25	Pass	0.69	Pass	0.09	Pass	0.97	Pass	0.13	Pass
Performance group PG-6													
16	1	0.61	Pass	0.27	Pass	0.74	Pass	0.19	Pass	0.95	Pass	0.27	Pass
17	2	0.61	Pass	0.27	Pass	0.74	Pass	0.19	Pass	0.95	Pass	0.27	Pass
18	4	0.61	Pass	0.27	Pass	0.74	Pass	0.19	Pass	0.95	Pass	0.27	Pass
Performance group PG-7													
19	1	0.53	Pass	0.26	Pass	0.34	Pass	0.11	Pass	0.42	Pass	0.15	Pass
20	2	0.75	Pass	0.42	Pass	0.47	Pass	0.15	Pass	0.49	Pass	0.19	Pass
21	4	0.87	Pass	0.57	Pass	0.72	Pass	0.24	Pass	0.87	Pass	0.27	Pass
Performance group PG-8													
22	1	0.59	Pass	0.30	Pass	0.47	Pass	0.23	Pass	0.60	Pass	0.32	Pass
23	2	0.63	Pass	0.36	Pass	0.64	Pass	0.40	Pass	0.65	Pass	0.38	Pass
24	4	0.87	Pass	0.60	Pass	0.73	Pass	0.46	Pass	0.92	Pass	0.58	Pass

ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Evaluate
Performance



Calibration of q_a
factor



ILLUSTRATIVE CASE STUDY EXAMPLE: QUANTIFICATION OF BEHAVIOR FACTOR FOR SUSPENDED PIPING SEISMIC RESTRAINT INSTALLATIONS

Document Results

In this last phase of the framework, the information on the behaviour of the analysed sway braced trapezes, the definition of the archetypes and their design, the development of the numerical models, the NLTHAs results, the performance objectives and the proposed design procedure should be summarized in a document to be submitted to a review panel.



CONCLUSIONS

1. Although significant efforts have been done in the last years to improve seismic performance of non-structural elements many efforts are still required to achieve the same level of knowledge available for structural systems.
2. A methodology for quantifying the seismic performance of non-structural elements has been proposed;
3. The methodology can be applied to many typologies of non-structural elements;
4. Few experimental/numerical data are available in the literature in order to apply the methodology, define performance objectives and improve design provisions.

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THANK YOU FOR YOUR ATTENTION



What the client wanted.



**The architect's
solution.**



**The structural engineer's
solution.**



**The non-structural engineer's
Solution.**

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